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Waste dump erosional landform stability – a critical issue for mountain mining

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Earth Surface Processes and Landforms

ABSTRACT: Mining is the largest producer of solid wastes which, when released to land or into waterways, can cause harmful environmental impacts. This is mostly due to fluvial erosion, which is highly increased in mountain areas, due to abrupt slopes. We have analysed this situation at a mountain watershed (192 ha), where steep mined sites and their waste dumps are the main source of sediment in a Natural Park. This problem was tackled by building gabion check dams downstream from the mined sites. We used the DEM of Differences (DoD) method to quantify erosion and sediment yield from three waste dumps (5 ha). Their topography and substrate properties were analysed to understand the erosion problem. The sediment trapped by the check dams was quantified by electrical resistivity tomography (ERT). The rainfall characteristics triggering an episode that filled the check dams with sediment in the winter 2009–2010, were studied to confirm whether it was a case of extreme precipitation conditions.

The waste dumps sediment yield $(353 \pm 95 \text{ Mg ha}^{-1} \text{ yr}^{-1})$ suggests severe landform instability. Analysis of topographic and substrate properties confirmed long, steep slopes combined with highly erodible materials. The check dams proved to be inefficient in controlling sediment loads, as they had only functioned for four years of 31 of existence, having trapped 13 000 ± 660 m³ of sediment, whereas we estimated that the waste dumps have yielded approximately three times more sediment for the same period. Rainfall analyses showed that neither intense nor extreme conditions (return period of 25 to 35 years) triggered the mobilization of $37 \pm 2 \text{ Mg ha}^{-1}$ in a month. This study highlights the fact that mining operations in similar mountainous settings, with equivalent waste dump construction and reclamation practices, are currently unfeasible. We conclude that landform stability cannot be achieved at this site without landform changes. Copyright © 2017 John Wiley & Sons, Ltd.

KEYWORDS: landform stability; mining erosion; check dams; electrical resistivity tomography (ERT); DEM of difference (DoD)

Introduction

The estimated annual global mine waste generation for 2011 was approximately 56 billion tonnes of waste rock and seven billion tonnes of tailings which is equivalent to about nine tonnes per human per year (Mudd and Boger, 2013). Mining is necessary for maintaining society's current life style and will continue to grow. However, mining can be detrimental to public health and safety, and may cause damage to the environment (on-site and off-site effects). This is due to mining-related removal of soils and vegetation and concomitant increased erosion, creation of large amounts of wastes and/or associated acid mine drainage (AMD) or increased suspended sediment loads in downstream waterways (Mossa and James, 2013). The generation of solid and liquid wastes and their release to land and into waterways are arguably the greatest impacts on the environment associated with mining (Franks, 2015). The export of runoff and sediment downstream (off-site effects) affects basic ecosystem services relating to water quality, fluvial wildlife conservation, recreation and flood control (Ehrenfeld, 2000), made worse by AMD.

In most situations, the root of the problem is the large amount of unconsolidated waste rock or tailing materials produced by mining operations that are accumulated near to the mining sites. They become new landscapes with a geomorphology often far from equilibrium and vulnerable to failures (Martín Duque *et al.*, 2010). Collectively they are referred to as waste facilities or structures, and individually as heaps, piles or dumps – for waste rock –, and dams or ponds – for tailings. The use of heaps, piles and dumps can differ according to specific characteristics and ore recovering potential (see Orman *et al.*, 2011), but they also refer to the same in different world regions. Here we will use 'waste dumps' (*escombreras* in Spain and *botaderos* in Ibero-America) to name the studied accumulations of waste rock, because it is the most common term in the geomorphic literature.

Landform instability of mining wastes rises sharply in mountainous physiographic settings, where long, steep natural slopes and narrow valleys impose difficult conditions for accumulation and stabilization. If other factors triggering instability are added to the constraints of mountains, the consequences can be dramatic in terms of effects on society and the environment (Table I). Catastrophes such as the examples shown in Table I have led to an improvement in geotechnical solutions. However, even in a context of broad geotechnical stability, waste structures are usually far from geomorphic equilibrium, often leading to accelerated erosion, even when they are reclaimed (we use here reclaimed or reclamation, although rehabilitation and rehabilitated are equivalent terms).

This study deals with erosion at mining waste dumps in mountainous regions and sediment mobilization downstream. Waste dump erosion is common in mountain mining and leads to the formation of alluvium deposits in downstream valley bottoms, thus inducing further environmental impacts in a much larger area than the one affected by the mining operations. Wohl (2006) characterizes the detrimental effects of this process on mountain streams: there is a sharp increase in sediment mobility, a decrease in bed and bank stability, and channel geometry is altered; water quality is degraded by sediment and other contaminants; reduced instream and riparian habitat availability and quality; and, finally, the disturbance regime is altered.

The largest set of scientific evidence of geomorphic-based environmental effects in waterways located downstream from

 Table I.
 Some catastrophes associated to landform instability in mountainous areas

Reported incidence and details	Reference
The negligent design of a coal waste dump triggered a collapse that killed 116 children and 28 adults (Aberfan, Wales)	Van Niekerk and Viljoen (2005)
Compilation of failures of tailings dams in the Andes Cordillera due to seismicity (Chile)	Villavicencio <i>et al.</i> (2014)
Perturbations on water flows and dramatic soil erosion lead to metal-rich sediment transport downstream into rivers and lagoon, due to nickel mountain mining (New Caledonia)	Bird <i>et al.</i> (1984); Losfeld <i>et al.</i> (2015)
Tailings dam failure at Ok Tedi mine and subsequent disposal of tailings into the Fly River (Papua New Guinea)	Franks (2015); Pickup (2001)
Tailings dam failure polluting the ocean (Placer Dome's mine, Marinduque Island, Philippines)	Franks (2015)

waste dumps comes from the so-called mountaintop removal mining (MRM), also mountaintop mining (MTM), of the Appalachian Coalfield Region of Eastern Unites States (Palmer *et al.*, 2010). In this region, mining performed within a physiography characterized by significant differences in height (several hundred metres) and steep valleys has induced fundamental changes in the topography and morphology of watersheds, has greatly affected the water quality of downstream ecosystems (Lindberg *et al.*, 2011) and led to heavier storm runoff and increased frequency and magnitude of downstream flooding (Phillips, 2004; Ferrari *et al.*, 2009; Miller and Zégre, 2014).

As already referred, on-site and off-site effects occur at reclaimed waste dumps even with geotechnical stability measures, a condition applying to most of today's mine waste dumps. This is key, since this affects economic viability, legal liability, bond release and successful ecological recovery. The reason is that waste dump resistance to fluvial erosion has not been solved satisfactorily. Artificially created topography may still be prone to erosion processes (Nicolau, 2003). At present, a growing body of literature is demonstrating that reclaimed waste dumps, even with geotechnical stability measures, are vulnerable to erosion (Haigh, 1979, 1980; Goodman and Haigh, 1981; Hahn et al., 1985; Riley, 1995; West and Wali, 1999; Evans et al., 2000; Evans and Willgoose, 2000; Kapolka and Dollhopf, 2001; So et al., 2002; Hancock et al., 2003; Hancock et al., 2008; Nyssen and Vermeerch, 2010; Merino-Martín et al., 2012; Beullens et al., 2014; Martín Duque et al., 2015). The most frequent mining reclamation approaches, based on terraced landforms and contour banks, tends towards erosion, especially via rill generation (Morenode las Heras et al., 2011) and the formation of gullies, owing to deficiencies in the functioning of berms and terraces during maximum rainfall events (Sawatsky et al., 2000). Inappropriate landform design has been claimed as a cause (Franks, 2015).

This broad meaning and request for landform stability is evident in the mine reclamation regulations of different countries (Table II), which means that the construction of stable post-mining landforms and erosion control are significant issues for many communities. Therefore, the purpose of this study is to describe and quantify erosional landform instability of waste dumps in a mountainous landscape, to improve mining operation and reclamation elsewhere.

Table II. The issue of landform stability as reflected in the mine reclamation regulations of different countries

Country	Landform requirements	Reference
Australia	Western Australia require safe, stable, non-polluting post-mining landforms. Queensland – rehabilitated areas are required to be safe for humans and wildlife, non-polluting, stable, and able to sustain an agreed post-mining land use	In: Howard <i>et al</i> . (2011, p. 112)
	end land use, that will not adversely affect surrounding land, and with maintenance needs no greater than the surrounding land.	
Canada	Achieve a viable and wherever practicable, self-sustaining, ecosystem that is compatible with a healthy environment and other human activities	Minister of Public Works and Government Services of Canada (1996, p. 14)
	Until self-sustaining vegetation cover is established, erosion control methods are necessary and include, among others, slope stabilization (p. 10) and will avoid unstable or steep slopes adjacent to watercourses or water bodies (p. 20)	Alberta Government (2005, p. 10 and 20)
Chile	Guarantee the physical and chemical stability of the sites where the extractive activities take place	Ministerio de Minería (2011, several pages)
USA	The reclaimed area, including any terracing or access roads, closely resembles the general surface configuration of the land prior to mining and blends into and complements the drainage pattern of the surrounding terrain	SMCRA (1977, p. 114)
	Provide a topography capable of stabilizing the surface from excessive erosion minimizing off-site effects and changes to the prevailing hydrologic balance	NMMMD (2010, p. 4)
Spain	Ensuring the stability of the waste and prevention of soil and water (surface and groundwater) pollution	RD975/2009, BOE (2009, several pages)

This study was carried out within the Poveda de la Sierra -Peñalén Mining District (east-central Spain), located at the edge of the Alto Tajo Natural Park (ATNP). This area comprises five main mining operations (three abandoned and two active) for kaolin (Figure 1). All five sites benefit the same materials (kaolin-rich quartz-feldspar sandstones of the Arenas de Utrillas Formation) and have the same topographical setting at the edge of high altitude plateaux. Mining operations here are very similar to those of the MRM or MTM of the Appalachian Mountains in the eastern United States. MRM or MTM is a form of surface mining, mostly for coal, at the summit of a mountain. Explosives are used to remove the top of summit ridges to expose underlying coal seams. At the kaolin mines of the Alto Tajo, explosives are employed in a limited way, only to remove the limestone cap lying at the edges of broad plateaux above the kaolin sands. Once the kaolin sands have been exposed, their bed fronts in the guarries are removed by hydraulic excavators and bulldozers.

Within this mining district, we studied the Peñalén watershed, which includes the Santa Engracia inactive mine, with two abandoned mined sites. The site was chosen because high landform instability at this location has been blamed as the main source of sediment yielded to the ATNP, and considered the main environmental problem in this protected area (Martín-Moreno, 2013). Mine instability led to a series of gabion check dams being built within the watershed from the 1980s to trap the sediment eroded in the mined areas and, as also stated, 'to "stabilize" both streams and slopes'. Therefore, the objectives of this paper are to: (i) study the landform stability of poorly designed and reclaimed outslope dumps of loose sands with clay of a kaolin mountain mine by measuring their erosion; (ii) quantify the sediment trapped by the downstream check dams, both for comparing erosion values at the dumps and to test their efficiency as a sediment control solution; (iii) study the precipitation characteristics that triggered a recent episode of filling the check dams storage; and (iv) develop better understanding of the mining system so that improved reclamation can occur.

Study Area and Context

Physical environment

The Peñalén watershed is located in the buffer zone of the ATNP, in east-central Spain (579535; 4502213, coordinate system UTM-30 N, datum ETRS 1989). This protected area was established in 2000 by a regional law (DOCM, 2000) because of its outstanding geodiversity and biodiversity, especially in regard to the aquatic ecosystems. This area is also included in the Natura 2000 Network site Alto Tajo Special



Figure 1. (A) Study area – Poveda de la Sierra – Peñalén Mining District location on the Iberian Peninsula. (B) Three-dimensional (3D) model showing the physiographic setting of the five main mined sites. (C) General view of the Santa Engracia mine (Path to Poveda site) in 1989. (D) General view of El Machorro mine in 2016. For C and D, note the similitude of this type of mining with the mountaintop mining (MTM) in the Appalachian Mountains in the eastern United States. [Colour figure can be viewed at wileyonlinelibrary.com]

Protection Area (SPA, ES0000092) and in the Comarca de Molina-Alto Tajo Geopark.

The Peñalén watershed covers an area of 192 ha; 173 ha are connected to the fluvial network and 19 ha cover endorheic watersheds at mine sites. The Peñalén stream, 1.7 km long and at a mean gradient of 14%, is a tributary of the Merdero stream, which in turn debouches into the Tagus (*Tajo*) River (Figure 2).

The Alto Tajo landscape is characterized by plateaux and mesas capped by Cretaceous carbonates (limestones and dolostones), in which the Tagus River has carved a canyon system over 100 km in length (Carcavilla et al., 2008). At the Peñalén watershed area, the plateaux have an altitude of approximately 1400 m above sea level (a.s.l.) and the mouth of the Merdero lies at 1025 m a.s.l. Underlying the carbonates is a sandy sediment that holds the kaolin (Arenas de Utrillas Formation) excavated in several mines (Olmo and Álvaro, 1989). On the plateaux and flat tops of mesas, the soils are chromic luvisols, calcaric cambisols, mollic leptosols and rendzic leptosol. On the slopes, carbonate colluvia with calcaric cambisols are common (IUSS Working Group WRB, 2007). The vegetation is representative of Mediterraneancontinental environments, with communities dominated by Juniperus thurifera on the highest plateaux and pine (Pinus nigra subsp. salzmanii) and gall oak (Quercus faginea) covering the valley slopes (MARM, 1997-2006). There are also some areas of non-irrigated arable lands. The climate of this area is temperate Mediterranean with dry, mild summers (Csb,

according to Köppen, 1918) but with a noticeable continental influence. The moisture regime is dry Mediterranean (Papadakis classification) (CNIG, 2004). Mean annual precipitation is 780 mm and mean annual temperature is 10°C (AEMET, 2016). According to the Thornthwaite method (Urbano Terrón, 1995) annual potential evapotranspiration is 679 mm. The seasons in this area are characterized by long, cold winters, commonly with snowfalls, and short dry summers with high intensity rainstorms. The spring and fall are usually wet. The rainfall erosive factor, *R*, is estimated to be about 800 MJ mm ha⁻¹ h⁻¹ yr⁻¹ (ICONA, 1988). This value is not very high, as *R* ranges from 350 to 5400 MJ mm ha⁻¹ h⁻¹ yr⁻¹ (ICONA, 1988) in Spain.

Mining activity in the Peñalén watershed

Since the eighteenth century, if not before, iron ore has been mined (in galleries) and manufactured in the Peñalén watershed. This activity had ceased by the end of the nineteenth century. In 1924 iron mines were re-opened and remained active until 1936, when the Spanish Civil War commenced (García López, 2012).

In the late 1960s, mining activity started again within the Peñalén watershed. This time the mineral of interest was kaolin and the mining method used mechanized open pits. Figure 3 shows the increase in size of the Santa Engracia mine (blue circles) between 1977 and 2009.



Figure 2. Peñalén watershed. CD = check dam. The Alto Tajo Natural Park lands are downslope of the green line, towards the Tagus River. [Colour figure can be viewed at wileyonlinelibrary.com]



Figure 3. Aerial photographs of the Peñalén watershed from 1946 to 2009. Both the vegetation cover and the mined sites have increased with time (see specifically aerial photographs from 1985 to 2009). Photograph sources: 1946 and 1957, Centro Cartográfico y Geográfico del Ejército del Aire (CECAF); 1977, 1985 and 1999, Instituto Geográfico Nacional (IGN); and 2009 Plan Nacional de Ortofotografía Aérea (PNOA) Ministerio de Fomento, Instituto Geográfico Nacional (IGN) and Centro Nacional de Información Geográfica (CNIG). The photographs are not georeferenced and show therefore some deformation. Central coordinates 579535E; 4502213 N, coordinate system UTM-30 N, datum ETRS 1989. [Colour figure can be viewed at wileyonlinelibrary.com]

The Cemetery mine site was opened in 1967; operation ceased after some years, but resumed again in 1980. In the same year, mining also began in the Path to Poveda site. At this location, three typical outslope waste dumps (as described by Haering *et al.*, 2004) were built on top of the original slope, downstream from the mine (Figure 4). These dumps are also called sidehill dumps (Orman *et al.*, 2011): a sidehill structure that lies along the side of a slope but does not cross a valley. Shortly after, work in the Cemetery mined site finished, but activity continued in the Path to Poveda site, until 1990.

In the 1990s, when mining activity ceased at the Path to Poveda site, the waste dumps, connected to the fluvial network, were poorly reclaimed – they were slightly regraded, by constructing a terraced topography and spreading colluvium on the benches. These terraces were severely eroded by gully erosion, triggering also mass movements. Revegetation works, consisting in planting various species of shrub, failed and natural plant colonization did not occur because of heavy erosion. Nowadays, only part of the original terraces are recognizable and they contain gullied incisions of up to 3 m (Figures 4, 5A and 5B).

Erosion has not only caused effects on-site, but off-site too. As stated earlier, the Santa Engracia mine sites have been identified as the most significant source of sediment in the ATNP (Figure 5D).

Check dam construction at the Peñalén watershed

The severity of erosion, from the Santa Engracia waste dumps, prompted the construction of a series of gabion check dams



Figure 4. Santa Engracia mine, Path to Poveda site, oblique aerial photographs, and ground pictures, spanning 30 years. In 1980, mining operations were active, and the process of outslope waste dumping can be clearly seen. In 1990 the waste dumps were slightly regraded, conforming a terraced topography, which eroded into a network of gullies (2010). 1980, 1989 and 1990 oblique aerial photographs by Paisajes Españoles S.A. [Colour figure can be viewed at wileyonlinelibrary.com]

in three periods (1981, 1984 and 2009–2010) as part of three separate Watershed Management Plans. Their purpose was to trap the sediments mobilized from the dumps to prevent transport to the Merdero stream and onwards to the Tagus River. In 1981, four 2-m high check dams were built in small watercourse tributaries of the Peñalén stream. In addition, a 4-m high check dam was built in the Peñalén stream, just before it joins the Merdero stream. In 1984, the storage areas of those check dams were filled by sediment and some were broken by debris flow, which formed debris cones within the valleys (Figure 5C). Following this, 4 m additional height was added to the existing 4-m check dam, to a total of 8 m in height [check dam number 2 (CD#2) in Figure 2]. The third Watershed Management Plan was carried out during the 2009–2010 winter; on this occasion the 8-m high check dam was partially cleaned and a new 4.5-m high check dam (CD#1 in Figure 2) was built upstream from the



Figure 5. Erosional and sedimentation landforms denoting very active processes at the Santa Engracia waste dumps. (A) Gully head retreat the waste dump B (see Figure 6 for specific location). (B) Detail of typical erosional and sedimentation landforms at waste dump A (see Figure 6 for specific location): straight gully cutting through the whole dump with an active alluvial fan at the toe. (C) A 2-m high check dam broken by a debris flow process, forming a debris cone (ellipse, which includes part of the dam); the dotted line shows the former position of the check dam, and the arrow shows the check dam displacement. (D) The Merdero stream emitting sediment into the Tagus River after a rainfall event, which main sources of sediment are the Santa Engracia mined sites, once the check dams are totally filled; Figure 2 shows the location of mined sites, check dams, drainage network and watersheds, and precise location of photograph D, downstream of the alluvial fan formed when the Merdero stream debouches into the Tagus River. [Colour figure can be viewed at wileyonlinelibrary.com]

previous one. In approximately one month (from 10 December 2009 to 18 January 2010), when CD#1 was under construction and had reached 3-m high, and CD#2 had been partially cleaned, their storage areas were completely filled by sediment. One month later, when CD#1 had reached 4.5-m high, its storage area was completely filled again.

Recent ecohydrological changes in Peñalén watershed

Water quality and the health of aquatic ecosystems in the Alto Tajo region have become worse in the last few decades as a consequence of mining development. The trout (*Salmo trutta*) population declined in the 1990s, as well as economic activity related to sports fishing, both of which concerned the Natural Park authorities. Kaolin mines were considered the main drivers of such environmental and economic problems, since sediment (sand, silt and clay) originating from the mines presumably covered the spawning areas in the beds of the watercourses, which in turn limited trout reproduction (Rafael Ruiz, former Director of Alto Tajo Natural Park, personal communication, 2007).

Mining in the area has not been the only cause of hydrological and ecological changes in these fluvial systems. Traditional land uses in the area, based on extensive ovine livestock, forestry and marginal agriculture, have also undergone a sharp reduction due to large numbers of people abandoning the countryside in the mid-twentieth century. As a consequence, a noticeable recovery of vegetation and wildlife is taking place, as shown in Figure 3. This is a general phenomenon occurring in Spain and other developed countries, with significant ecological effects, such as a decrease in water flowing along watercourses and a reduction in soil erosion (Rey Benayas *et al.*, 2007). Fauna species are also recovering, including some fluvial species of trout predator, such as the otter (*Lutra lutra*) and the cormorant (*Phalacrocorax carbo*).

Although sediment production in the Peñalén watershed is a complex and a changing process with several drivers, the Santa Engracia mine is undoubtedly the major source of sediment in this watershed, causing the most adverse environmental effects to the ATNP (Martín-Moreno, 2013).

Materials and Methods

Waste dump topography and substrata characterization

In order to study the erodibility of waste dump materials, nine composite samples of substrata were collected from the gullies and intergully areas of the three waste dumps (three samples for each waste dump). Each sample was formed by six subsamples distributed at random, which were homogeneously mixed to minimize spatial variability. General physico-chemical characteristics were determined: particle size distribution was measured by the pipette method (Gee and Bauder, 1986) and using the US Department of Agriculture (USDA) textural classification; mineralogical composition was determined by X-ray diffraction (XRD) using a Bruker D8 ADVANCE diffractometer with Cu-Ka ($\lambda = 1.54$ Å) radiation, and the relative amount of every phase was determined following the procedure by Chung (1974a, 1974b). The content of sodium was measured by inductively-coupled plasma optical emission spectrometry (ICP-OES). Prior to analysis, samples were dissolved in deionized water, shaken during 24 hours and filtered; pH and electrical conductivity (EC) were measured at 25°C by a CRISON pH-meter basic 20+ and a CRISON EC-meter Micro CM-2200, respectively; finally, organic matter (OM) was estimated by the loss-on-ignition method (Netherlands Normalisation Institute, 1994). Additionally, bulk density was calculated by the core method (Blake, 1965). We took nine unaltered samples with an 8.2 cm high by 8.2 cm diameter core. Mean bulk density values were calculated for each waste dump and from the nine samples. Slope length and gradient were measured by topographical CAD analyses.

Quantification of waste dump erosion

To quantify the volume of eroded material at the three waste dumps (letters A, B and C, Figure 6), we first built a digital elevation model (DEM) representing the detailed topography of the eroded waste dumps (recent situation or 2009-DEM). To build this DEM, we used the most recent high resolution public airborne light detection and ranging (LiDAR) data available (provided by the Spanish National Plan for Aerial Ortophotography, PNOA), with a minimum density of 0.5 points m⁻² and a root-mean-square error in z (RMSEZ) of 20 cm (according to the technical specifications, PNOA, 2009). From this DEM, 1-m contours were generated and the area occupied by each waste dump delineated. Then, we erased the contours within the gullies, leaving only the parts representing the original terraces. Next, the contours of the original terraces were edited to reconstruct the original terraced topography. The oblique aerial photographs taken in 1990, where terraces are visible (see Figure 4), were used as a reference. Field identification of the remains of the original terraces, preserved in the intergully areas, were also used. Following that, we built another DEM representing the terraced topography without gullies (1990-DEM). For this DEM we also assumed a RMSEZ of 20 cm equal to the raw point cloud.

Finally, both DEMs, the original reconstructed terraced, 1990-DEM, and the one representing a recent situation with



Figure 6. Three-dimensional (3D) view of the current appearance (2016) of the Path to Poveda mined site. The dotted lines delineate the three studied waste dumps. (Image courtesy of the Consejería de Economía, Empresas y Empleo, Junta de Comunidades de Castilla-La Mancha). [Colour figure can be viewed at wileyonlinelibrary.com]

gullies, 2009-DEM (Figure 7), were used to produce a DEM of difference (DoD) by using the Geomorphic Change Detection (GCD) software (GCD, 2014). DoD measures the net change corresponding to erosion and deposition budgets (Wheaton et al., 2010). In this case, the volume of waste eroded since the terracing landform construction, from 1990 to 2009. DoD uncertainty was estimated specifying a minimum level of detection threshold (minLoD) which let us distinguish real geomorphic changes (above minLoD) from noise (below minLoD). Additionally, GDC software uses this minLoD to calculate the error of the volume of waste eroded, which is estimated, in turn, from those areas where real geomorphic changes can be detected. The minLoD was calculated assuming that individual errors of each DEM - the current (2009-DEM) and the original reconstructed terraced (1990-DEM) - are propagated, as expressed in the following equation (adapted from Wheaton et al., 2010):

$$\delta u_{DoD} = \sqrt{(\delta z_{2009-DEM})^2 + (\delta z_{1990-DEM})^2}$$

where $\delta u_{\text{DoD}} \delta u_{DoD}$ is the propagated error in the DoD, and $\delta z_{2009 - DEM}$ and $\delta z_{1990 - DEM}$ are the individual errors for 2009-DEM and 1990-DEM, respectively. The error is assumed to be 20 cm in both cases. Hence, the propagated error used as minimum level of detection threshold (minLoD) was ± 28 cm.

The mass of eroded waste (in megagrams) was calculated by multiplying the volume (in m^3) by the waste bulk density. We also measured the projected area of each waste dump by using the 2009 orthophoto (PNOA, 2009), and ArcGIS 10.2 software. The mean annual total erosion (equivalent here to sediment yield, since no sedimentation occurs within the gullies) for each waste dump, in terms of Mg ha⁻¹ yr⁻¹, was calculated by dividing the mass of waste (in megagrams) by the area of



Figure 7. Digital elevation models (DEMs) of the Path to Poveda mined site waste dumps (named with the same letters A, B, C as in Figure 6). Top (1990), reconstructed DEM; bottom, situation in 2009, DEMs built from available LiDAR data (PNOA 2009). (Coordinate system UTM-30 N, Datum ETRS 1989.) [Colour figure can be viewed at wileyonlinelibrary.com]

each dump (in hectares) and the 19-year time span in which the recorded erosion took place (1990–2009).

Quantification of sediment trapped by the check dams

To calculate the volume of sediment trapped by CD#1 and CD#2, we used electrical resistivity tomography (ERT). ERT is a common geophysical technique used to investigate subsurface geometry and to characterize ground materials in mineral exploration, archaeology and environmental studies, among others (Dentith and Mudge, 2014).

The geophysical parameter used to characterize the ground is resistivity, which expresses the resistance that the ground opposes to direct current (d.c.) electrical currents. Ground resistivity (in Ω m) is related to various geological parameters, such as the mineral content, porosity and degree of water saturation in the substrata (Dentith and Mudge, 2014). Multiple electrodes are driven into the ground and connected by a multi-core cable with a resistivity-meter and power supply. The d.c. current is supplied to two electrodes (current electrodes) and voltage differences are measured between another two electrodes (voltage electrodes). Apparent resistivity (ρ_{ap}) value is calculated from the following equation (Reynolds, 2011):

$$\rho_{\rm ap} = K \frac{\Delta V}{I}$$

where ΔV is the potential difference between a pair of electrodes, *I* is the current intensity introduced into the ground, and *K* is the geometric factor which depends on the distances between voltage and current electrodes.

Usually an array with a large number of equidistant electrodes is used and a resistivity value is calculated by each four-electrode location. The injection and voltage electrodes are displaced automatically along the line to obtain resistivity values for a depth level. When all the resistivity values have been calculated with the same electrode separation, the distance between current and voltage electrodes is increased to calculate resistivity at a deeper level. This process is repeated until the vertical section is completed. The depth of investigation is limited by the electrode array length, as well by the current used (typically the total depth of penetration is approximately 20% of the electrode array length).

Two-dimensional (2D) data comprise pseudo-depth maps which may be displayed as images or contours, but the true form of the subsurface electrical structure usually is not easily recognizable. As a consequence, inverse modelling is an integral part of resistivity interpretation (Reynolds, 2011). The inversion is based on finite-element modelling, with a routine that calculates synthetic apparent resistivity data as a function of pseudo-depth, for a true resistivity-depth model. Elevation data should also be used to correct for irregular topography at the start of the inversion process. The process is repeated automatically for either a set number of iterations or until the percentage root-mean-square error (RMSE) falls below an acceptable value.

The interpretation of data measured is in two steps: firstly the physical interpretation, obtaining a physical model; and secondly the geological interpretation of this model (Dahlin, 2001; Muñoz Martín *et al.*, 2007).

Five ERT profiles were made in July 2012; two at the storage area of CD#1 and the remaining three at the storage area of CD#2 (Table III and Figure 8). We used the RESECS DMT system with 48 electrodes separated at 2.5 m and a Wenner–Schlumberger array. We performed two injection cycles with reversed polarity and 120 V. Over 250 values of voltage and intensity current were captured for each injection, and the error and variation in the standard deviation for each mean measurement were calculated. To obtain the real resistivity distribution at depth and its subsequent geological interpretation, we used the RES2DINV code inversion method (Loke and Barker, 1996) with four-node trapezoidal finite elements. Finally, we interpreted the geology over the 2D resistivity models and identified contact between substrata and sediment trapped by the check dams.

To calculate the volume of sediment trapped by the check dams, we assigned *XYZ* coordinates to each ERT profile by using a global positioning system (GPS) on 1:1000 cartography and a tape measure. We also assigned the coordinates to each electrode and to each thickness value of sediment infill. Then, we added the mapped infill limit (each check dam storage area) as a boundary condition (thickness = 0 m) and a regular net of $1 \text{ m} \times 1 \text{ m}$ was interpolated by the minimum curvature method in order to build the infill isopach map of each check dam storage area.

To quantify the mass of sediment, we multiplied the calculated volume (in m^3) by the sediment bulk density (in g cm⁻³), obtained by the core method (Blake, 1965). We took five samples at the surface of the storage areas behind CD#1 and CD#2.

Analysis of rainfall and its relation to the check dam filling process

Between 10 December 2009 and 18 January 2010, a period of continued precipitation caused by winter cold fronts occurred in this area. This front, which included rain and snow, triggered erosion and mobilized sediment that filled the check dams storage. The rainfall characteristics of this period and the volume of sediment trapped by the check dams for the same period were correlated. Rainfall was registered by a tipping-bucket automatic rain gauge (0.2 mm pulse⁻¹) (Davis Instruments, 2005) with a HOBO Event data logger. From these data, we calculated the total rainfall volume (in millimetres) for this period, its return period in years (*T*), the maximum rainfall volume in 24 hours (in millimetres), the maximum rainfall volume in 30 minutes (I_{30} , mm h⁻¹). To calculate the return period,

Table III. Characteristics of the electrical resistivity tomography (ERT) profiles performed at the storage areas of check dam number 1 (CD#1) and number 2 (CD#2)

ERT profiles		Number of electrodes	Length (m)	Levels of investigation	Depth reached (m)
CD#1	ERT 1	18	42.5	9	8
	ERT 2	32	77.5	12	11
CD#2	ERT 3	35	85	14	13
	ERT 4	48	117.5	14	14
	ERT 5	16	37.5	7	7



Figure 8. (A) Location of the electrical resistivity tomography (ERT) profiles (Coordinate system UTM-30 N, Datum ETRS 1989); (B) field measurements; (C) detail of an electrode and multi-core cable. [Colour figure can be viewed at wileyonlinelibrary.com]

the historical data series from Peñalén, Zaorejas and Beteta weather stations were analysed with the CHAC software (CEDEX, 2004). The general extreme values (GEVs) frequency distribution was used with maximum likelihood estimation (MLE) method. The return period was estimated graphically using the GEV curves calculated for each station.

The volume of sediment trapped by the check dams for this same period was extrapolated from the ERT results. To calculate the volume we took into account that on 10 December 2009, CD#1 was under construction (unfinished), standing only 3 m high. By that date, CD#2 had been partially cleaned; more specifically, a 1-m depth of the sediment trapped behind the dam had been removed from the storage area (Figure 9). We used Surfer 11 software to calculate the volumes. Once again, the volume (in m^3) obtained was multiplied by the mean bulk density to obtain the mass of sediment (in megagrams). Knowing the Peñalén watershed area connected to the fluvial network (173 ha), we computed the sediment yield for 10 December 2009 to 18 January 2010, in terms of Mg ha⁻¹ (approximately in one month).

Results

Waste dump topography and substrata characterization

Table IV shows the topographic and substrata physico-chemical properties of the three studied waste dumps, which have long, steep slopes (from 50 to 127 m of length and from 55% to 60% gradient). Texture is sandy loam with high percentage of sands (> 72%) and low proportion of silt (10.6% mean) and clays (13.6% mean). Mineralogy basically comprises quartz (~70%), kaolinite (9.3–20.3%), calcite (1.7–7.3%), dolomite (1–2.7%), mica/illite (4–6.3%), k-rich feldspar (1.3–3.7%), ankerite, plagioclase and hematite. The sodium content, EC and OM are very low. The pH values vary from 8.63 to 8.91. Finally, the mean bulk density is 1.54 ± 0.04 g cm⁻³.

Quantification of waste dump erosion

The volume eroded at waste dump (A) (see Figure 6) from 1990 to 2009, was 10886 \pm 2827 m³, equivalent to 16764 \pm 4354 Mg. For an area of 2.64 ha and for 19 years, mean annual erosion was 334 \pm 87 Mg ha⁻¹ yr⁻¹. For waste dump (B), the eroded volume was 7827 \pm 1649 m³, equivalent to 12054 \pm 2539 Mg. Its mean annual erosion (for 1.34 ha) was 473 \pm 100 Mg ha⁻¹ yr⁻¹. Finally, 3455 \pm 1513 m³ (5321 \pm 2330 Mg) were eroded at waste dump (C), so the mean annual erosion (for 1.11 ha) was 252 \pm 110 Mg ha⁻¹ yr⁻¹. In total, 22168 \pm 5989 m³ (34 139 \pm 9223 Mg) were eroded at the Santa Engracia waste dumps. This yields a mean annual erosion over the 19 years of 353 \pm 95 Mg ha⁻¹ yr⁻¹ (Table V).

Quantification of sediment trapped by the check dams

According to the ERT results, the thickness of the sediment trapped by CD#1 reached a maximum of 5 m and had higher resistivity than the substrate (Figure 10). This is because the substrate consisted of well-sorted sands and fairly compact (weakly cemented) sandstones (Arenas de Utrillas Formation), while the trapped sediments were more porous, unconsolidated sand, gravel and carbonate rock fragments, which translates into higher resistivity (> 160Ω m, Figure 10). In the case of CD#2, the maximum thickness of the trapped sediment was about 9 m (Figure 10). The gradient of the contact surface between the trapped sediment and the substrate is constant upstream from the check dam, but varies sharply close to the check dam. CD#2 was built on carbonate materials, so in this case, the sediment presents resistivity values lower than the substrate (< 100Ω m, Figure 10). In both cases, the sediment closest to the check dams has low resistivity (< 40Ω m), and can be as little as 10Ω m, typical of 'pure' clays.

The positioning error of ERT profiles is estimated at 1 m for XY and 0.5 m for Z; therefore the error in sediment thickness



Figure 9. Check dam storage areas before and after the 10 December 2009–18 January 2010 period. Top: CD#1 under construction (3 m high) and CD#2 partially cleaned; middle: check dam storage areas completely filled by sediment; bottom: 4.5 m high CD#1 and CD#2 some months after the studied period, their storage areas were completely filled by sediment. [Colour figure can be viewed at wileyonlinelibrary.com]

estimations is about 0.5 m. The error and variation in the standard deviation for each ERT profile were < 10% and the final 2D resistivity models had RMSEs below 5%, hence the calculated volume uncertainty assumed is about 5%. Consequently, the total volume of sediment trapped by CD#1 was approximately 2700 ± 140 m³, and 10300 ± 520 m³ by CD#2. The mean bulk density of the trapped sediment calculated was 1.57 ± 0.03 g cm⁻³, therefore the equivalent mass of sediment was 4200 ± 210 Mg for CD#1 and 16200 ± 810 Mg for CD#2 (see Table VI).

Sediment trapped by CD#1 came from material eroded between 10 December 2009 and 18 January 2010 (when the check dam was only 3-m high) and from sediment trapped after 18 January 2010, once the check dam was finished and stood 4.5 m high. According to the workers who carried out the last Watershed Management Plan, the storage area upstream of CD#1 was totally filled within one month after 18 January 2010; therefore, it can be said that this storage area was completely filled in two months.

Sediment trapped by CD#2 has a more complex temporal interpretation, because it had been trapped from 1981, when CD#2 was built, to 2012, when the ERT was performed (31 years). In addition, it must be borne in mind that during the last Watershed Management Plan, this storage area was partially cleaned but filled again in approximately one month (10 December 2009–18 January 2010).

Analysis of rainfall and its relation to check dam filling

The total rainfall volume for the period 10 December 2009–18 January 2010 was 351 mm. The maximum rainfall volume in

Table IV. Topography and substrata physico-chemical properties of the three waste dumps

		Waste dump				
	Units	А	В	С		
Topography						
Maximum slope length	m	127	91	50		
Slope gradient (mean)	%	55	60	58		
Area	ha	2.64	1.34	1.11		
Substrata characteristics						
Mineralogy	%	quartz 71.0 ± 3.6	quartz 69.7 ± 11.8	quartz 71.0 ± 8.7		
		kaolinite 9.3 ± 3.2	kaolinite 20.3 \pm 15.0	kaolinite 16.3 ± 3.2		
		calcite 7.3 ± 9.3	calcite 1.7 \pm 2.1	calcite 1.7 ± 1.2		
		dolomite 2.0 ± 1.0	dolomite 2.7 ± 0.0	dolomite 1.0 ± 0.0		
		ankerite 2.3 \pm 2.5	mica/illite 4.0 ± 3.5	plagioclase 2.0 ± 2.0		
		mica/illite 4.3 ± 1.5	K-feldspar 1.3 ± 0.0	mica/illite 6.3 \pm 5.9		
		K-feldspar 3.7 ± 0.0	hematite 0.3 ± 0.0	K-feldspar 1.3 \pm 0.0		
				hematite 0.3 ± 0.0		
Clay (< 0.002 mm)	%	16.2 ± 0.05	12.3 ± 0.03	12.4 ± 0.03		
Silt (0.002–0.02 mm)	%	11.3 ± 0.01	7.9 ± 0.03	12.7 ± 0.04		
Fine sand (0.02–0.2 mm)	%	31.1 ± 0.06	31.5 ± 0.05	32.8 ± 0.06		
Medium sand (0.2–0.5 mm)	%	19.4 ± 0.03	23.5 ± 0.05	21.1 ± 0.05		
Coarse sand (0.5–2 mm)	%	22.0 ± 0.03	24.8 ± 0.05	21.0 ± 0.06		
Texture	USDA	Sandy loam	Sandy loam	Sandy loam		
Bulk density	g cm ⁻³	1.55 ± 0.07	1.57 ± 0.11	1.50 ± 0.13		
pH		8.63 ± 0.09	8.91 ± 0.06	8.75 ± 0.12		
Sodium content (in solution)	$\mu g g^{-1}$	76.0 ± 3.5	89.3 ± 23.6	6.6 ± 2.8		
Electrical conductivity (EC)	dS m^{-1}	0.09 ± 0.04	0.06 ± 0.01	0.06 ± 0.00		
Organic matter (OM)	%	0.43 ± 0.17	0.37 ± 0.11	0.40 ± 0.06		

Note: mean \pm standard deviation values are shown. Number of samples = 3.

Table V. Volume and mass of sediment eroded at the waste dumps of Santa Engracia mine since its regrading, 1990-2009, for a minLoD = ± 28 cm

Waste dump	Area (ha)	Area of detectable change (ha)	Volume of eroded waste (m ³)	Error (%)	Mass of eroded waste (Mg)	Annual erosion-sediment yield (Mg ha ⁻¹ yr ⁻¹)
A	2.64	1.04	10886 ± 2827	± 26	16764 ± 4354	334 ± 87
В	1.34	0.58	7827 ± 1649	± 21	12054 ± 2539	473 ± 100
С	1.11	0.54	3455 ± 1513	± 44	5321 ± 2330	252 ±110
Total	5.09	2.16	22168 ± 5989		34139 ± 9223	353 ± 95^{a}

Note: It also includes the calculated error, the total values.

^aThe sediment yield of each waste dump and weighted mean annual sediment yield.

24 hours registered during this period was 49 mm and the maximum rainfall volume in 60 minutes and in 30 minutes were 9.4 and 12 mm h^{-1} , respectively. The return period, graphically estimated by using the historical data from Beteta, Zaorejas and Peñalén weather stations, was between 25 and 35 years (Figure 11).

For the studied period, CD#1 trapped $1100 \pm 60 \text{ m}^3$ (1700 \pm 90 Mg) of sediment. The recently cleaned storage area behind CD#2 was filled by a volume of $3000 \pm 150 \text{ m}^3$, equivalent to $4700 \pm 240 \text{ Mg}$. To sum up, from 10 December 2009 to 18 January 2010 both check dams trapped a total sediment volume of about $4100 \pm 210 \text{ m}^3$, or approximately $6400 \pm 330 \text{ Mg}$ (Table VI). Taking into account that the area of Peñalén watershed connected to the fluvial network is 173 ha, sediment yield was about $37 \pm 2 \text{ Mg ha}^{-1}$ in one month.

Discussion

Erosion rates and check dam sediment control

The mean annual erosion (equals to sediment yield here), 353 \pm 95 Mg ha⁻¹ yr⁻¹, is in the same order of magnitude as values measured in other severely eroded mine sites, most of them on

sandy mine wastes without reclamation (Table VII). However, sediment supply from the Santa Engracia waste dumps to the fluvial network is likely to have been higher than those calculated, as according to the miners and to the Peñalén neighbours (personal communication, 2010), the waste dumps underwent intense erosion processes, including mass movements, during their construction (when check dams did not exist).

These values and facts imply severe erosion conditions and therefore, high landform instability. The reasons for such intense erosion in this location are not related to run-on entrance and are numerous:

i the waste dumps were built with relatively easily erodible materials (loose sands, > 72%, with clay and silt, sandy loam texture, and a low percentage of OM); in fact, as directly observed in the field, surface erosion on these waste materials occurs virtually after every rainstorm. The low values of EC and content in sodium, and the lack of evaporite minerals (salts), indicated that Santa Engracia waste dumps are not susceptible to tunnel erosion and dispersive behaviour – processes usually described for sodic and saline soils (So and Aylmore, 1993) –, so that we interpret that the main factor influencing the waste materials erodibility is the particle size distribution (Hancock *et al.*, 2008) – sandy



Figure 10. (A) Two examples of electrical resistivity tomography (ERT) profiles. Notice the contact between the sediment trapped by the check dams and the substrata. Also observe the fine facies settled upstream from the check dams; (B) check dam storage area isopach maps. [Colour figure can be viewed at wileyonlinelibrary.com]

Table VI. Volume and mass of sediment trapped by check dams

	Total (198	1–2012)	Period 2009–2010		
Check dam	Volume of sediments (m ³)	Mass of sediments (Mg)	Volume of sediments (m ³)	Mass of sediments (Mg)	
CD#1	2700 ± 140	4200 ± 210	1100 ± 60	1700 ± 90	
CD#2	10300 ± 520	16200 ± 810	3000 ± 150	4700 ± 240	
Total	13000 ± 660	20400 ± 1020	4100 ± 210	6400 ± 330	

loam textures are prone to rill and gully erosion, and fine sand content, high here, increase significantly erodibility. In addition, the low OM content could favour erosion due to difficulties for aggregate formation.

- ii The construction technique was a direct waste dumping process on long, steep natural slopes.
- iii Waste dumps had single linear long, steep slopes (maximum slope length up to 127 m and mean gradient up to 58%), which is a critical issue in the development of erosion processes (combined with high erodible materials).
- iv Ineffective reclamation measures to control erosion and sedimentation were applied.

The results demonstrate a very low efficiency of the check dams. The volume of material eroded at the waste dumps over 19 years (1990–2009) was 22 168 \pm 5989 m³ and the total volume of sediment trapped by the check dams in 31 years (1981–2012) was 13000 \pm 660 m³. Therefore, at least 9168 m³ of sediments could have reached the Merdero stream and, in turn, the Tagus River. The eroded volume for 1990 to 2009 (19 years) can be interpolated from the rainfall record for the 1981–2012 period (31 years). This shows that the material eroded at the waste dumps for the latter period would be 36169 m³, and the volume supplied to the Tagus 23169 m³, which is approximately 2–3 times more, respectively, than the



Figure 11. General extreme values (GEV) curves of (A) Beteta, (B) Peñalén and (C) Zaorejas weather stations. Continuous lines represent the total rainfall volume (351 mm) for the period 10 December 2009–18 January 2010, and broken lines the estimated return period (years). [Colour figure can be viewed at wileyonlinelibrary.com]

sediment trapped by the dams. Additionally, the check dam life span was expected to be several decades; however, they were filled in a much shorter time.

According to Rafael Serrada (manager of the 1980s Watershed Management Plans, personal communication, 2008) the check dams built in 1981 were totally filled in approximately three years, and CD#2, raised during the second

Watershed Management Plan in 1984, was filled in about one year. As stated by the workers who built the check dams in 2009–2010, these were filled in two months. This means that the check dams have had a lifespan of about four years. In short, the use of check dams as a structural solution for controlling the impact of mining was unsuitable. The reasons are that they: (i) were inefficient as a sediment control measure; (ii) had, in themselves, a high environmental impact, given their artificiality and ecological effects in the fluvial systems of a protected area; (iii) posed erosion problems for their surroundings arising from their construction and the building of access roads, as reported elsewhere (Brandt, 2000); and (iv) were very expensive.

A clear conclusion arises here. It is much more efficient (and ecological) to concentrate on stabilizing the sediment source area than trying to control and store the sediment yielded by check dams. Should the Peñalén watershed have no check dams and stably reclaimed mined areas, it would not have: a segmented longitudinal fluvial profile; an altered sediment dynamics; free longitudinal movement of nutrients and aquatic organisms (Wohl, 2006), all of which are important functions within a protected area.

The last episode of check-dam filling (between 10 December 2009 and 18 January 2010) was not the result of any extreme event, but just normal conditions. The total volume of rainfall registered had a return period as 25-35 years and recorded intensities were not very high. The maximum intensity in 60 minutes for this period was 9.4 mm h^{-1} , classified as moderate according to AEMET (2015). We interpret this as erosion and sediment yield for this period not being activated by high intensity rainfall, but due to the effect of continuous precipitation, which saturated the material. Saturation not only greatly favours runoff, it also causes loss of cohesion in materials, which can trigger mass movements that mobilize materials from the slopes to the bottom of the valley (Garland and Olivier, 1993).

All the measured evidence indicated that waste dump erosion at this location has been, and remains, very high. Thus, once the check dams are already totally filled, the current sediment emission from the mined areas to the fluvial network of the ATNP remains, continuing the loss of ecosystem services, as a consequence of externalized costs by unsustainable mining practises.

Implications for contour and/or mountain mining viability

Steep and/or mountainous terrain is constrained by 'lack of space' to dispose waste. Valleys fills have been, and are, a common solution for disposing or accommodating waste in mountainous terrains, and always have a high environmental impact.

This study highlights the fact that proper stabilization and reclamation of waste dumps should be a critical issue for mining in mountainous terrain. In these settings, effective control, from the short to the long term, of on- and off-site effects and minimizing the damaging environmental impacts caused by these, is a must. This is more critical when, in addition, mountain mining takes place in the vicinity of protected areas.

Mountain mining implies the presence of long, steep natural slopes, where accumulation of unconsolidated mine waste often have intrinsically high vulnerability to erosion and to mass movements. If weather conditions are also prone to causing high erosion, common in mountainous areas, the waste dump landform stability becomes a critical issue.

We demonstrate that waste (hillside) dumps built by direct dumping on nearby slopes, which has been a common

Table VII.	Comparison of waste	dump sediment	yield obtained	in this study	y with sediment	yields measured	at other mine sites
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Study site	Description – Study scale	Methodology	Published values	Calculated values. Sediment yield (Mg ha ⁻¹ yr ⁻¹)	Sources
Santa Engracia mine, Peñalén (Guadalajara, Spain)	Terraced waste dumps built on overburden materials	DoD	а	353 ± 95	This study
AMAX Ayrshire Mine, Evansville, Indiana (EEUU)	Relationship between gradient slope and erosion	Rainfall simulations in erosion parcels	67.8 Mg ha ⁻¹ 117.7 Mg ha ⁻¹ 144.3 Mg ha ⁻¹		Hahn <i>et al.</i> (1985)
Eastern Oklahoma (EEUU)	Slope or ground retreat on abandoned spoil banks	Erosion pins	7.6–9.5 mm yr ⁻¹	160–180	Goodman and Haigh (1981)
Gallia County, Ohio (EEUU)	Abandoned waste dumps. Sandy soil	Erosion parcels	а	203	West and Wali (1999)
Ohio County, Kentucky (EEUU)	Terraced waste dumps. Subsoil.		а	116	
Kidston Gold Mine, North Queensland (Australia)	Slope scale. Waste dumps without vegetation cover.	Erosion parcels	а	425	So et al. (2002)
San Cristóbal–Perules mining, Mazarrón (Southeast Spain)	Terraced slope of a tailings dam	DoD	a	152	Martín Duque <i>et al.</i> (2015)
El Machorro mine, Guadalajara (East-Central Spain)	Linear slope built with overburden (sandy materials)	Erosion parcels with sediment fences	а	347	Martín-Moreno <i>et al.</i> (2016)

Note: The values of erosion rates refer to mean values or value ranges published in each case. The values in bold were calculated using a density of 1.5 g cm^{-3} , the mean for sandy soils (Porta *et al.*, 1999).

^aPublished value coincides with that shown in the sediment yield column.

practise, gives rise to high instability. Reshaping, by building terraces and by spreading colluvium on benches, was not sufficient to stabilize the facilities, and was just a 'makeup' measure. The difficulties in reaching long-term stability, but also for finding space to dispose the waste under these physiographic conditions, have been reported for the Appalachian Mountains. The enactment of the previously mentioned SMCRA in the United States required all mining spoil to be stabilized and returned to the 'approximate original contour' (AOC) (Brenner, 1985). For the mountain mining of the Appalachia, this meant that the extensive highwall cuts had to be covered and returned to contour, filled with spoils. But several authors (Brenner, 1985; Bell et al., 1989; Zipper et al., 1989) demonstrated that the use of this AOC requirement in these steeply sloping topographies, led to common slope instability, excessive mining costs, increased erosion and loss of value in post-mining land use. The main reason for these failures was that this type of AOC reclamation cannot replicate the original geological structure and soil properties, and that steep topographies on unconsolidated mining wastes lead to general instability.

Building stable waste dumps is therefore critical, but in order to find solutions, understanding and quantifying the stability of mined landforms (reclaimed or not) in real scenarios of steep slopes is essential. Especially if they have had a few years (preferably decades) of geomorphic evolution, as in the case we report here.

In search of durable landform stability of mine waste dumps

In the last two decades, a growing number of researchers and practitioners have identified landform instability as a limiting factor for successful mining reclamation, soil development and revegetation (references as in the Introduction section). The development of valuable new tools to improve landform performance has grown at the same time. This is the case of erosion models, such as a specific version of USLE (Universal Soil Loss Equation) for evaluating erosion in mined sites, RUSLE 1.06 (Toy and Foster, 1998); the SOILOSS model, an adaptation of USLE developed in Australia (Rosewell and Edwards, 1988); the WEPP (Water Erosion Prediction Project, Nearing *et al.*, 1989), applied in mined areas by Elliot *et al.* (1993); the adaptation of the Thornes model by Moreno-de las Heras et al. (2011); also the MUSLE approach (Modified Universal Soil Loss Equation, Williams, 1975) fitted to mined sites as well (Fifield, 2001). Landscape Evolution Models (LEMs), mostly SIBERIA (Willgoose *et al.*, 1989) have been widely used in this context (Hancock *et al.*, 2008, 2016), but also CAESAR (Coulthard *et al.*, 2002). There is also a fluvial geomorphic landform design method (GeoFluv) with its CAD-based software (Carlson Natural Regrade) that has been largely applied in the United States, whose potential use as a solution for mining reclamation in mountainous areas is being evaluated (DePriest *et al.*, 2015). Any of these methods and models have potential use here.

We suggest a series of landform guidelines for stabilizing and reclaiming the kaolin mountain mined sites of Santa Engracia: (a) lowering the gradient of the linear slopes of the three main waste dumps, by removing materials at their upper reaches; (b) adopting a natural catchment approach for reclamation, regrading mature smooth valleys in these gentle slopes until slope and channel stability is reached; (c) the excess of materials cut at the upper reaches of waste dumps and in the valleys would partially fill the pit between the highwall and external waste dumps, building also mature stable natural catchments; (d) finally, the remaining outcropping upper part of the highwall could be reshaped mimicking analogous natural landforms for the physiographic setting, such as stable cliffs. Once the foundations for landform stability have been laid, additional and indispensable soil replacement and revegetation measures should complete a holistic ecological approach. Ecological recovery is not likely without landform stability.

About methodology

The methodology used to calculate erosion at the waste dumps has certain limitations. Firstly, although LiDAR data is of high quality and meets international standards (ASPRS, 2014), the point cloud density used to obtain the 2009-DEM (recent situation) is not sufficient to represent the complex topography of the gully system developed on the waste dumps. In addition, field evidence indicates that some of the intergully areas are not in their original position, as they have been eroded and are lower than the original terraced topography. Secondly, the degree of uncertainty of the reconstructed topography (1990-DEM) of the interpreted original waste dumps is presumably high. Thirdly, we assumed that individual DEMs' errors were the RMSEZ – 20 cm – but solely because it is the only error available and reported in the technical specifications of the raw point cloud (PNOA, 2009). Although this reliability was not checked in the field (as recommended, Wheaton *et al.*, 2010), it is like other errors reported for LiDAR data used with GCD software which can range between 11 and 26 cm (Schaffrath *et al.*, 2015). Finally, the volume of eroded sediment calculated by using the DoD methodology has a highly uncertainty, ranging from $\pm 21\%$ to $\pm 44\%$ (see Table V).

Despite these limitations and uncertainties, we believe that the methodology used is an acceptable approach to estimate waste dump erosion.

The ERT profiling used to calculate volume and mass of sediment trapped by the check dams also presents errors and uncertainties. Estimated errors relating to the position or resolution of ERT are below 0.5 m. Nevertheless, the main source of uncertainty is the lack of three-dimensional (3D) information to assess the full geometry of the dam infilling. This absence has been solved by obtaining longitudinal, transverse and oblique tomographic sections along the dams. The incorporation of the mapped limit of the check dam storage area as a boundary condition allowed us to calculate more realistic volumes. This methodology guarantees that surface contact between substrata and sediment trapped by the check dams is identified correctly, thus obtaining the geometry of the storage areas with a good degree of precision (Ahmad *et al.*, 2009).

Regarding rainfall analysis and its relation to filling the check dams, the main limitation is that we only registered rainfall, but not snow. We already know that, during the studied period, snow precipitation also occurred, in some occasions with rain on snow, causing rapid melting and heavy sediment mobilization. The geomorphic relevance of such a process has been described, for instance, in the central Pyrenees (López-Moreno and García-Ruiz, 2004).

Despite these limitations, we consider that the methodology used allowed reliable data to be obtained to make a quantified diagnosis of the described problem.

Conclusions

Historically, the Peñalén watershed in east-central Spain has been subject to natural high erosion processes and sediment fluxes into the Tagus River, including areas (such as gullies) that have been the source of a significant amount of sediment yield. This high geomorphic activity is conditioned by the extremely strong energy of the watershed relief, the existence of erodible geological outcrops (kaolin sands and clay) and weather erosivity factors, as shown by active gullying and shallow slides. This 'natural' erosion has been part of the dynamics of the watershed over the last few centuries, with variations associated with land-use changes (affecting vegetation cover). Despite this, the fluvial ecosystems have adapted to these conditions, because of the slow rate at which land use changed. However, this situation was severely disrupted by intensive mechanized opencast mountain mining activities in the watershed, starting in 1967.

Mining operated without considering the high vulnerability of the physiographic setting for landform stability and took no account of stable landform designs, which led to very high erosion-sediment yield from the Santa Engracia waste dumps $(353 \pm 95 \text{ Mg ha}^{-1} \text{ yr}^{-1}, \text{ mean}).$

Once this landform instability became evident, in 1981, check dams were adopted as a solution, to avoid unacceptable sediment yields to pristine fluvial ecosystems. Check dams, in addition to having an intrinsically high environmental impact, were very inefficient at storing the sediment eroded within the watershed.

This study, one of the few that reports erosion rates in mountain mining, highlights the fact that this type of mining is inviable, here and elsewhere, with such poor waste dump landform stability consideration and reclamation practices. Therefore, new ways of mining operation and reclamation, as an integrated process, need to be developed in such contexts. After having quantified the erosion and sediment yield from three waste dumps, and having analysed their topography and substrate properties, we also conclude that any action aimed at 'restoring' here balanced runoff and sediment fluxes should be based on sound geomorphic solutions, rebuilding functional natural catchments, with stable slopes and drainage networks, implying therefore landform changes that must be completed by additional and indispensable soil development and revegetation measurements. We believe that the conclusions of this work can be used for better environmental performance in mountain mining, avoiding undesirable situations like the one described.

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